Flat Salb Dynamic Analysis

¹ Nalini Thakre, ² Mahesh Janbandhu, ³ Dipak Mangrulkar

^{1, 2, 3} S.B.Jain College of engineering, Nagpur University, Department Of CIVIL Engineering Nagpur, Maharashtra

Abstract - The seismic effect on the structure is the most important factor while designing the civil Engineering structures which will cause adverse effect later on. In present study efforts being taken while analyzing the structure for Earthquake resistance. In this we have taken a live project a five storied building with flat slab and dynamic analysis is done by Response Spectrum Method taking all load combinations as per IS-1893. We have used software SAP 2000 for this purpose. Main emphasis is given on the structural response when flat slab is provided. Behaviour of the structure is studied for four types of models like a flat slab only, Flat slab with head, with drop and with both head and drop with equal thickness as well unequal thickness. The results of all four types are then compared for time period, axial forces, shear forces, moments in each direction and the slab panel moments. The structure is designed as per IS-456-2000 and IS-13920 guidelines.

Keyword - Response spectrum Method, SAP-2000, IS CODES-1893, 456-2000, 13920.

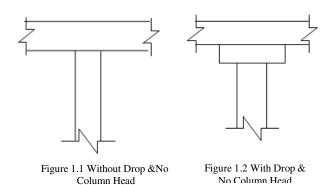
1. Introduction

1.1 General

Flat slab is a reinforced concrete slab supported by columns with, or without drops. The columns may be with, or without, column heads. Flat-slab is one of the most widely used systems in reinforced concrete construction because of its high degree of structural efficiency. It use simple formwork and reinforcing arrangements, and requires the least storey height. Although efficient in resisting gravity load, the flat slab system is inherently flexible and can have excessive lateral drift when subjected to seismic loading. Its susceptibility to severe damage during strong earthquakes is well documented (Rosenblueth 1986; Hawkins 1980).

In zone of high seismicity, the flat-slab systems are designed such that slab-column space frame supports gravity loads and the shear walls provide resistance to lateral load (Wey and Durani 1992; Robertson and Durani1992; Moehle and Diebold 1985). However, it is required by the building codes [IS: 456; ACI: building

1989] that the gravity load subsystem must be able to deform with the lateral load resisting system without any loss of its load carrying capacity. Thus, in realty the two subsystems act together. Furthermore, since the design seismic force recommended by the codes are generally much less than what the structure would experience during a major earthquake, a certain degree of nonlinear response is to be expected.



1.2 Advantages

Some of the advantages of Flat Slab are:-

- 1. Rapid construction
- 2. Maximum design flexibility
- 3. Economy
- 4. Headroom height requirements large
- 5. Minimum storey heights
- 6. Controlled deflection
- 7. Optimum clear span
- 8. Sufficient lateral displacement capacity

1.3 Terminology Related With Flat Slab

1.3.1 Column Strip

Column strip means a design strip having a width of 0.25 l_1 , but not greater than 0.25 l_2 , on each side of the column

centerline, where I_1 , is the span in the direction moments are being determined, measured centre to centre of supports and l_2 , is the-span transverse to l_1 , measured centre to centre of supports.

1.3.2 Middle Strip

Middle strip means a design strip bounded on each of its opposite sides by the column strip.

1.3.3 Panel

Panel means that part of a slab bounded on-each of its four sides by the centre-line of a column or centre-lines of adjacent-spans.

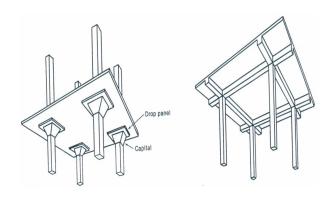


Figure 1.4 Flat slab with drop & column Head Structure

Figure 1.5 Slab Beam Structure

2. Analysis of Flat Slab by Response Spectrum Method

2.1 Introduction to Problem

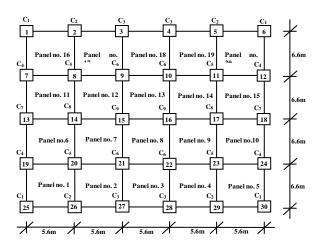
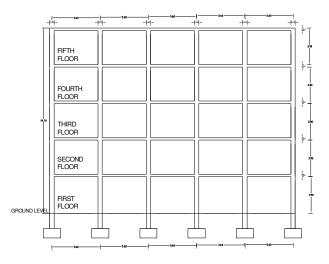


Fig.2.1Line Plan of Structure



2.2 Modeling of flat slab for Response Spectrum

- Column size (500x500)mm
- Slab thickness 200 mm without drop
- Live load on all floors 6 kN/m² (50% for earthquake)
- Grade of concrete M25 & steel Fe 415N/mm²
- Brick wall external & internal 230 mm
- Density of brick wall including plaster 20kN/m³
- Density of brick wall including plaster 20kN/m³
- Drop size external (1100x1000)mm
- Drop size internal (2200x 2000)mm
- Depth of Head 400mm
- Head at bottom (500x500)mm
- Head at top (1200x1200)mm

2.2.1 Mathematical Model

Table No. 2.1 Mathematical Model

Sr.No	Perticulars			
1	The mathematical model includes five storey			
	building with 30 column in each floor and 20			
	slab section			
2	All columns are modeled as beam element and			
	slab is modeled as plate element having area			
	type shell			
3	Each plate is sub divided into 9*8 =72 plate			
	element			
4	For dynamic analysis of structure response			
	spectrum method was used			

2.2.2 Soil Strata

It is assumed that the footing is resting at 1.5m from GL

Medium soil

2.2.3 Seismic Force Calculation

Zone II for Nagpur city Z=0.1 Hence spectra for medium soil \

Importance factor =1.5

 $Sa/g=1+15T0.00 \le T \le 0.101$

Damping 5%

= 2.5 $0.1 \le T \le 0.55$

Soil type medium = 1.36/T $0.55 \le T \le 4.0$

R for SMRF=5 & OMRF=3

- 1. Time period in long direction $Tx = 0.09H/\sqrt{d}$ =0.09x21/ $\sqrt{28}$ =0.357sec
- 2. Time period in short direction Ty=0.09H/ \sqrt{d} =0.09x21/ $\sqrt{26.4}$ =0.368sec

Take Sa/g for both direction = 2.5

2.2.4 Scale Factor

$$f = \frac{1}{2} x \frac{I}{R} = 0.15$$

Where, I = Importance factor = 1.5

R=Response reduction factor=5

2.2.5 Response Spectrum for Zone II:-Soil Type II (Medium Soil)

Table No. 2.2 Time Period Vs Acceleration

Period(Sec)	Acceleration	
0	0.1	
0.1	0.25	
0.55	0.25	
0.8	0.17	
1	0.136	
1.2	0.1133	
1.4	0.0971	
1.6	0.085	

1.8	0.0756
2	0.068
2.5	0.0544
3	0.0453
3.5	0.0389
4	0.034
4.5	0.034
5	0.034
5.5	0.034
6	0.034
6.5	0.034
7	0.034
7.5	0.034
8	0.034
8.5	0.034
9	0.034
8.5	0.034
10	0.034

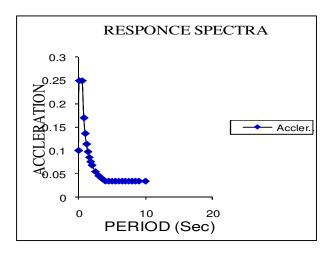


Figure 2.6 Response Spectra for Zone II

689

806

876

ISSN: 2348 - 6090 www.IJCAT.org

C7

C8

C9

2.3 Results and Discussion

715

838

869

2.3.1 Comparison of Time Period (sec)

TYP FLAT DROP WITH WITH Ε **SLAB** DROP **HEAD HEAD** C1 593 573 581 569 C2728 708 716 699 C3 708 689 695 681 **C**4 722 703 712 693 C5 819 825 809 836 C6 833 810 821 799

708

827

888

695

815

879

TIME PERIOD FOR ALL FLAT SLABS (Sec)					
MODES	FLAT	HEAD	DROP	DROP HEAD	
1	1.60	1.61	1.65	1.66	
2	1.56	1.57	1.61	1.62	
3	1.47	1.50	1.53	1.53	
4	0.47	0.47	0.49	0.49	
5	0.46	0.46	0.48	0.48	
6	0.43	0.43	0.44	0.44	
7	0.23	0.23	0.24	0.42	
8	0.23	0.23	0.23	0.24	
9	0.21	0.21	0.21	0.23	
10	0.14	0.13	0.14	0.23	
11	0.14	0.13	0.14	0.21	
12	0.12	0.12	0.12	0.21	

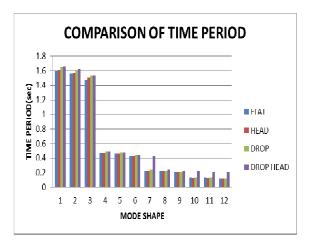


Fig. 2.7 Comparison of Time Period (sec)

Description & Interaction (Time Period)

As it is seen from SAP-Results there are many modes out of which if we consider the first two modes it has been seen that the time period of flat slab without Head & Drop seems to be low due to which Sa/g will be less and lateral forces will also be less

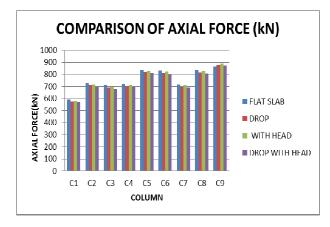


Fig.2.8 Comparison of Axial Force (kN) First Storey

Description & Interaction (Axial Force)

As we go from fifth Story to the first story we can see the first story have long column and we have maximum axial force to be developed in the column of first story with drop and head.

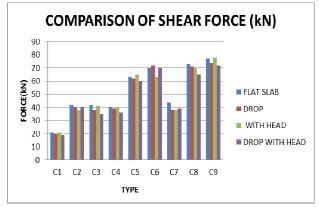
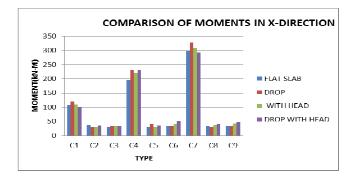


Fig.2.9Comparison of Shear Force (kN) First Storey

Description & Interaction (Shear Force)

From SAP Result it is seen that due to lateral forces developed are in decreasing order from flat to drop with head & there is not much differences in the models with only drop & only head.



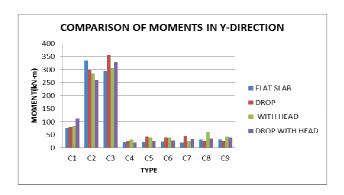


Fig2.10Comparison of Moment in X- Direction (kN-m)

Description & Interaction (Moments)

From the graph it is found that the moments are directly proportional to the stiffness. Since the stiffness is going to increase from flat to drop with head & if we compare Slab with the head & Slab with the drop. The stiffness will not very much there fore the moment variation are not much.

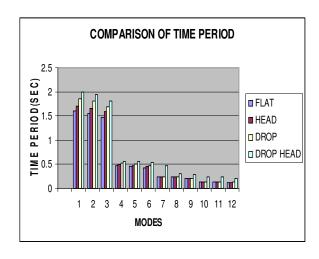


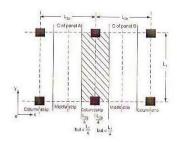
Fig. 3.1 Comparison of Time Period (sec)

4. DESIGN OF FLAT SLAB BY IS 456:2000

	FLA'	T DROP		P	WITH		DROP	
	SLA	В			HEAD		WITH	
							HEA	D
TYP	M2	M3	M2	M3	M2	M3	M2	M3
E								
C1	76	107	80	120	84	110	112	101
C2	335	38	300	30	285	30	261	35
C3	295	31	357	32	306	32	330	32
C4	22	196	25	231	32	221	21	231
C5	23	30	45	41	40	31	27	35
C6	24	32	40	32	40	41	28	51
C7	21	298	46	328	25	307	34	293
C8	31	32	25	31	62	37	36	41
C9	31	32	26	32	45	42	40	48

The term flat slab means a reinforced concrete slab with or without drops, supported generally without beams, by columns with or without flared column heads. A flat slab may be solid slab or may have recesses formed on the soffit so that the soffit comprises a series of ribs in two directions. The recesses may be formed by removable or permanent filler blocks.

4.1 Components of flat slab design



4.1.1 Column Strip

Column strip means a design strip having a width of 0.25 I_1 , but not greater than $0.25l_2$, on each side of the column centre-line, where I, is the span in the direction moments are being determined, measured centre to centre of supports and I_1 , is the -span transverse to I_2 , measured centre to centre of supports.

4.1.2 Middle Strip

Middle strip means a design strip bounded on each of its opposite sides by the column strip.

Type	Main	Main	Stirrups	Stirrups
	R/F	R/F	IS 456-2000	IS 13920-
	IS 456-	IS		1993
	2000	13920-		
		1993		
C1	6-	6-	8mmDia.	1)8mmDia.
	25Ф+2-	25Ф+2-	2L@180mm	2L@250mm
	16Ф	16Ф		
				2)8mmDia.
				3L@80mm

4.1.3 Panel

Longer span	Shorter span
L_1 =6.6m, L_2 =5.6m	L_1 =5.6m, L_2 =6.6m
(i) column strip	(i) column strip
= 0.25 L_2 = 1.65m	= 0.25 L_2 = 1.4m
But not greater than 0.25	But not greater than
L_1 = 1.4 m	0.25 L_1 = 1.4m
(ii) Middle strip	(ii) Middle strip
=6.6-(1.4+1.4)=3.8m	=5.6-(1.4+1.4)=2.8m

Panel means that part of a slab bounded on-each of its four sides by the centre -line of Column or centre-lines of adjacent-spans.

Division into column and middle strip along:

Drop dimensions along:

Table 4.6 Steel Calculation

4.7 Design Of column by IS-456-2000 and IS-13920

Detailing of C3 Column for First Storey

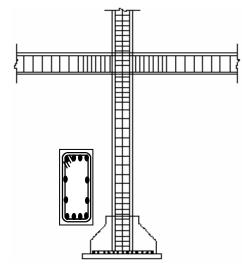


Figure 4.2 Detailing of C3 column as per IS 13920:1993

Table 4.1 Span Calculations

4.1.4 Drop

When provided shall be rectangular in plan, and have a

Panel	Longer span	Shorter span	
Column strip	-956kN-m	-568kN-m	
Middle strip	+134 kN-m	+104 kN-m	
Column strip	-956 kN-m	-956 kN-m	
Reinforcement	4664mm^2	3512mm ²	
-ve steel	1938mm^2	1508 mm^2	
Reinforcement	20mm \(\phi \) @ 130c/c	20mm \(\phi \) @ 130c/c	
+ve steel Spacing -ve	16mm φ @ 100c/c	16mm ϕ @ 130/c	
steel			
Spacing +ve steel			

length in each direction not less than one- third of the panel length in that direction. For exterior panels, the width of drops at right angles to the non- continuous edge and measured from the centre-line of the columns shall be equal to one-half the width of drop for interior panels. Since the span is large it is desirable to provide drop.

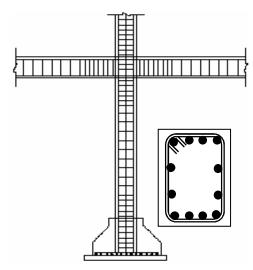


Figure 4.3 Detailing of C1 column as per IS 13920:1993

5. Conclusions

- 1. Drops are important criteria in increasing the shear strength of the slab.
- 2. By incorporating heads in slab, we are increasing rigidity of slab.
- 3. The dynamic analysis results indicate that the lowest mode of vibration i.e. third mode was the torsional mode. This seems to be a typical of flat-slab building with a central core of shear walls.
- 4. Modeling of flat plate slab with diaphragm and without diaphragm in case of response spectrum method there is no variation in axial force, shear force and moment as moment of inertia of slab is very high it acts as rigid.
- 5. The negative moment's section shall be designed to resist the larger of the two interior negative design moments for the span framing into common supports.
- 6. Enhance resistance to punching failure at the junction of concrete slab & column by providing drop with head.
- 7. Drop with head & Flat slab head is very good combination to reduce the moment with less thickness of slab.
- 8. In earthquake zone we shall provide only flat slab drop with head & ductile detailing for all structure.

References

- [1] Abdel Wahab, El-Ghandour, Kypros Pilakoutas, and Peter Waldron, "Punching Shear Behavior of Fiber Reinforced Polymers Reinforced Concrete Flat Slabs: Experimental Study.", Journal of Composites for Construction, Vol. 7, No.3, August 1, 2003.
- [2] ACI Manual of Construction Practice 1995 Part 3 Page No. 318/318R-(201-228).
- [3] 3.Ahmed Ghobarah, "Performance-Based Design in Earthquake Engineering: State Of Development", Engineering Structures Elsevier Science 23,874-884, March 17, 2001.
- [4] Austin P.D, Moehle J.P, "Interior Slab Column Connection of Building," 2000
- [5] B. A. Izzuddin, X. Y. Tao; and A. Y. Elghazouli, "Realistic Modeling of Composite and Reinforced Concrete Floor Slabs under Extreme Loading.", I: Analytical Method Journal of Structural Engineering, Vol. 130, No. 12, December 1, 2004.
- [6] Criteria for Earthquake Resistant Design of Structures Part 1 General Provisions and Buildings (Fifth Revision) IS 1893(part 1):2002.
- [7] Dr. Ram Chandra, Virendra Gahlot, "Elements of Limit State Design of Concrete Structures", Scientific Publishers (India), 2004.
- [8] Dr. R.K. Ingle "Excel Spreadsheet for Design of Colum by IS 456-2000 & IS 13920", Applied Mechanics Department, VNIT, Nagpur.
- Ductile Detailing of Reinforced Concrete Structures to Seismic Forces IS 13920:1993

- [10] Durrani A. J.; Mau S. T., AbouHashish A. A., and Yi Li, "Earthquake Response of Flat-Slab Buildings", Journal of Structural Engineering, Vol. 120, No. 3, March, 1994.
- [11] Ehab El-Salakawy, Khaled Soudki, and Maria Anna Polak, "Punching Shear Behavior of Flat Slabs Strengthened with Fiber Reinforced Polymer Laminates", Journal of Composites for Construction, Vol. 8, No. 5, October 1, 2004.
- [12] Indian standard "Plain & Reinforced Concrete Code of Practice" Fourth Revision IS: 456:2000.
- [13] Joo-Ha Lee, Young-Soo Yoon, Seung-Hoon Lee, William D. Cook, and Denis Mitchell, "Enhancing Performance of Slab-Column Connections", Journal of Structural Engineering, Vol. 134, No. 3, March 1, 2008.
- [14] L. Cao and C. J. Naito, "Design of Precast Diaphragm Chord Connections for In-Plane Tension Demands", Journal of Structural Engineering, Vol. 133, No. 11, November 1, 2007.
- [15] M. Altug Erberik and Amr S. Elnashai "Loss Estimation Analysis of Flat-Slab Structures", Natural Hazards Review, Vol. 7, No. 1, February 1, 2006.
- [16] M.Anitha, B.Q.Rahman, JJ.Vijay, "Analysis and Design of Flat Slabs Using Various Codes", International Institute of Information Technology, Hyderabad (Deemed University) April 2007.